

### **LANSERIA AIRPORT EXTENSION 1** SOUTHERN PRECINCT

### STORMWATER MANAGEMENT REPORT

REPORT 2019-094-33 Rev-1 **NOVEMBER 2025** 

### **CLIENT: GROWTHPOINT PROPERTIES, APETURE PROPERTIES** & LANSERIA INTERNATIONAL AIRPORT



PREPARED BY:

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Lynnwood, Pretoria

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Tel (012) 991 1205 Fax (012) 991 1373

#### **Outline Scheme Report Information Sheet**

Report number : 2019-094-33-Rev-1

Local authority : City of Johannesburg Metropolitan Municipality

Zoning type : Special

Property description : Lanseria Extension 1, Southern Precinct

Report undertaken by:

Name : D.H. van der Merwe

Signature :

Qualifications : B.Eng (Civil)

Email address : <u>dean@edseng.co.za</u>

Report reviewed by:

Name : F.H.B van Eyk Pr. Eng

Signature :

Qualifications : B.Eng (Civil), B.Eng (Hons) (Water Resources)

ECSA Registration : Pr. Eng. 20160826

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## LANSERIA AIRPORT EXTENSION 1 – SOUTHERN PRECINCT STORMWATER MANAGEMENT REPORT

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#### 1 INTRODUCTION

EDS Engineering Design Services (Pty) Ltd (EDS Engineers) was appointed to compile a Stormwater Management Report (SWMR) for a portion of Erf 183 Lanseria Airport Extension 1 referred to as the southern precinct of Lanseria Airport Extension 1

The application site is located in Lanseria and falls in the area of jurisdiction of the City of Johannesburg Metropolitan Municipality.

This report describes the existing civil engineering stormwater management in proximity to the application site, the expected demands as a result of the development under the existing land-use rights and evaluates the proposed stormwater networks to accommodate the expected demands from the southern precinct of Lanseria Airport Extension 1.

#### 2 DETAILS OF THE APPLICANT

The details of the applicant:

Company Name: GROWTHPOINT PROPERTIES

Physical Address: The place

1 Sandton Drive

Sandton Gauteng 2196

Contact Person: Polla Scholtz

Telephone Number: (011) 944 6050

Company Name: APERTURE PROPERTIES

Physical Address: 89 Bute Lane, Sandown

Sandton Gauteng 2196

Contact Person: Hilton Carty

Telephone Number: (079) 916 3982

Company Name: LANSERIA INTERNATIONAL AIRPORT

Physical Address: Airport Road

Lanseria 1748

South Africa

Contact Person: Trevor Teegler

Telephone Number: (012) 809 2229

#### 3 SITE INFORMATION

#### 3.1 SITE LOCATION

The site is located within the municipal boundaries of the City of Johannesburg.

The site details are as follows:

Site		Erf 138 Lanseria Airport Extension 1 – Southern Precinct
Size		25.5 ha
Boundaries	North	Lanseria Extension 1 - Runway
	East	Extension 11 & 12
	West	Lanseria Ext 35 RE/5/530-JQ
	South	Lanseria Ext 75

The location of the proposed township is shown in **Annexure A**.

#### 3.2 PROPERTY DESCRIPTION

The application site is on the southern precinct portions of Lanseria Airport Extension 1.

The physical combined size of Southern Precinct applicable to the SWMR of Lanseria Airport Extension 1 is 25.5 ha.

For the purposes of this report, the site will hereafter be referred to as Southern Precinct of Lanseria Airport Extension 1.

The proposed layout is included in **Annexure B**.

#### 4 DEVELOPMENT INFORMATION

#### 4.1 EXISTING LAND-USE RIGHTS

Erf 183 Lanseria Airport Extension 1 is currently zoned "Special" permitting land use for purposes necessary and in connection with airport, including aircraft hangers, aircraft maintenance, storage of goods, and accessories related to aircraft maintenance offices which are related to the use of the erf and buildings for the purposes of aircraft operations, including shops, retail and place of refreshment facilities subservient to the main use of the erf (which shall not exceed 2 500m²), aircraft runways, taxi aprons, air traffic control towers, repair and maintenance facilities, and such other associated land uses which the municipality may approve in writing.

A copy of the land use rights certificates is included in **Annexure C**.

Table 4.1.1: Existing land use rights

Erf nr	Existing zoning	Size (ha)	Density	Coverage	FAR	Height
Lanseria Airport X1 Southern Precinct	Special	25.5	N/A	N/A	0.2	5 Storeys

#### 5 OBJECTIVES OF THE STORMWATER MANAGEMENT PLAN

The objectives of the stormwater management plan are as follows:

- To determine the stormwater runoff for the pre-development site conditions.
- To determine the stormwater runoff for the post-development conditions.
- To ensure that the stormwater runoff for a 100-year storm can discharge through a flood escape route to ensure that no flooding on site occurs.
- To ensure that the quantity and the rate of stormwater runoff from the site is controlled as per the requirements of the Johannesburg Roads Agency (JRA).
- Calculate and allow for external stormwater management as no external system is available in the facility of the proposed development.

#### 6 PLANNING AND DESIGN CONSIDERATIONS

#### 6.1 REQUIREMENTS OF THE JOHANNESBURG ROADS AGENCY (JRA)

The Stormwater Management Report and design considerations are based on the requirements of the following policies and design guidelines:

- The City of Johannesburg Metropolitan Municipality *Stormwater Management By-Law*.
- Johannesburg Roads Agency SOC Limited (JRA) Roads & Stormwater Manual Volume 1 Code of Procedure.
- The South African National Roads Agency SOC Limited Drainage Manual
- Stormwater design manual for the City of Johannesburg 2019.

#### 7 CURRENT STORMWATER SYSTEM

The existing stormwater reticulation consists of the following:

• There are no municipal stormwater infrastructure networks in the surrounding area of the application site.

Stormwater information obtained from the Johannesburg Roads Agency (JRA) is included in **Annexure D**.

A natural watercourse is located to the northeast of Lanseria Extension 11. The southern precinct of Lanseria Extension 1 drains via overland flow into this watercourse.

Additionally, Lanseria Extension 75 includes a stormwater attenuation dam, which discharges overland onto the southern precinct of Lanseria Extension 1. From there, the stormwater continues to drain toward the aforementioned natural watercourse.

Lanseria Extension 1 Approved Stormwater management plan indicated two attenuation dams but these dams were never constructed. The unfinished areas created low points were stormwater accumulated and saturated the natural ground. The stormwater discharge from Lanseria Extension 75 also accumulated in these unfinished attenuation areas of Lanseria Extension 1.

The existing site stormwater management flow route is illustrated in **Annexure E** and the approved stormwater management plan included in **Annexure F**.

#### 8 PROPOSED STORMWATER SYSTEM

The Stormwater Management By-Laws of the City of Johannesburg mandate that an on-site stormwater drainage facility be provided for every development site. Such facilities must be designed with sufficient capacity to convey stormwater without flooding or causing damage to any existing or proposed structures.

A new stormwater system will be installed for Lanseria Extension 11 (X11) to accommodate upstream stormwater runoff originating from Lanseria Extension 1 (X1) Southern Precinct and Lanseria Extension 75 (X75). This system has been designed to accommodate a 1:100-year flood event, as previously approved as part of the township application for Lanseria X11. This system will hereafter be referred to as the Stormwater Connection Point X11. For detailed culvert design specifications, refer to the flood line study in **Annexure G**.

The new Stormwater Management Plan (SWMP) for Lanseria X1 Southern Precinct will consist of an integrated network of underground stormwater systems, ultimately connecting to the Stormwater Connection Point X11. On-site stormwater attenuation facilities will be implemented to ensure that post-development runoff rates are reduced to match pre-development runoff rates. The main stormwater culvert system will convey both the upstream runoff from Lanseria X75 and the stormwater generated within Lanseria X1 Southern Precinct to the connection point. The proposed stormwater network is included in **Annexure H**.

Lanseria X75 is an approved township and is required to comply with the City of Johannesburg's Stormwater Management By-Laws. Stormwater attenuation facilities within X75 include outlet control structures that regulate discharge to prevent downstream flooding. Lanseria X75 existing attenuation pond positions are included in **Annexure E**.

Lanseria X1 Southern Precinct will feature an underground piped stormwater system, designed to convey stormwater into the proposed attenuation facilities. These facilities will control outflow to the main culvert system, which leads to the Stormwater Connection Point X11.

It is important to note that attenuation facilities are generally not recommended at airports, as they attract birdlife, posing a risk to aviation safety. As a mitigation measure, the proposed attenuation facilities will incorporate a concrete-lined channel to direct stormwater efficiently to a controlled outlet structure, thereby preventing standing water and reducing the likelihood of bird attraction.

#### 9 DESIGN CALCULATIONS

The area of the application site for the stormwater calculation is **25.5 ha**.

#### 9.1 ESTIMATED PRE-DEVELOPMENT STORMWATER RUNOFF

The simplified rational method was used to determine the estimated pre-development runoff for the application site. The design data for determining the stormwater runoff for the 2- to 50-year recurrence interval design storm events for the pre-development conditions are summarised in **Table 9.1.1**.

Table 9.1.1: Pre-development runoff data

		Lanseria Extension 1 Southern Precinct	
Catchment Area (A)		255 000	m²
MAP		750	mm/year
Runoff Factor (C)		0.3	
Time of Concentration (Tc)		52	minutes
	1-2 year	35	mm/hr
	1-5 year	49	mm/hr
Painfall Intensity (I)	1-10 year	60	mm/hr
Rainfall Intensity (I)	1-20 year	72	mm/hr
	1-25 year	77	mm/hr
	1-50 year	92	mm/hr

The following formula applies:

$$Q = CIA/3.6$$

Where,

Q = Peak flow (m<sup>3</sup>/s) C = run-off coefficient

I = Average rainfall intensity over the catchment (mm/hr)

A = Catchment area  $(m^2)$ 

The estimated pre-development stormwater runoff for the 2- to 50-year recurrence interval design storm events is summarised in Table 9.1.2 below.

Table 9.1.2 Estimated pre-development stormwater runoff

		Lanseria Extension 1 Southern Precinct	
	1-2 year	0.37	m³/s
	1-5 year	0.57	m³/s
Peak Flow (Q)	1-10 year	0.77	m³/s
reak riow (Q)	1-20 year	1.03	m³/s
	1-25 year	1.14	m³/s
	1-50 year	1.62	m³/s

The pre-development stormwater calculations are included in **Annexure I**.

#### 9.2 ESTIMATED POST-DEVELOPMENT STORMWATER RUNOFF

The simplified rational method was used to determine the estimated post-development runoff for the application site. The design data for determining the stormwater runoff for the 2- to 50-year recurrence interval design storm events for the post-development conditions are summarised in **Table 9.2.1**.

Table 9.2.1: Post-development runoff data

		Lanseria Extension 1 Southern Precinct	
Catchment Area (A)		255 000	m²
MAP		750	mm/year
Runoff Factor (C)		0.7	
Time of Concentration (Tc)		15	minutes
	1-2 year	80	mm/hr
	1-5 year	107	mm/hr
Painfall Intensity (I)	1-10 year	129	mm/hr
Rainfall Intensity (I)	1-20 year	152	mm/hr
	1-25 year	157	mm/hr
	1-50 year	183	mm/hr

The following formula applies:

$$Q = CIA/3.6$$

Where,

Q = Peak flow  $(m^3/s)$ 

C = run-off coefficient

I = Average rainfall intensity over the catchment (mm/hr)

A = Catchment area (m<sup>2</sup>)

The estimated post-development stormwater runoff for the 2- to 50-year recurrence interval design storm events is summarised in **Table 9.2.2**.

Table 9.2.2 Estimated post-development stormwater runoff

		Lanseria Extension 1 Southern Precinct	
	1-2 year	3.97	m³/s
	1-5 year	5.31	m³/s
Peak Flow (Q)	1-10 year	6.40	m³/s
reak riow (Q)	1-20 year	7.54	m³/s
	1-25 year	7.78	m³/s
	1-50 year	9.07	m³/s

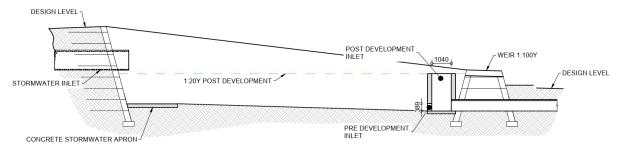
The post-development stormwater calculations are included in **Annexure I**.

#### 9.3 ATTENUATION PONDS

From Tables 9.1.2 and 9.2.2 above, there would be an increase in the stormwater runoff for the pre-development conditions of the application site.

The runoff associated with the development is to be attenuated such that the predevelopment flow for the 5- to 25-year storm events is not exceeded. The attenuation facility must also be capable of withstanding the 50-year storm event.

The proposed typical attenuation pond detail is shown below, the final drawings will be submitted with the Site Development application for approval to local authorities.



TYPICAL ATTENUATION POND SECTION

#### 9.3.1 SIMPLIFIED HYDROGRAPH METHOD

The simplified hydrograph method, as stipulated in the SANRAL Drainage Manual was used to calculate the estimated required stormwater attenuation volume for the application site and is summarised in Tables 9.3.1.

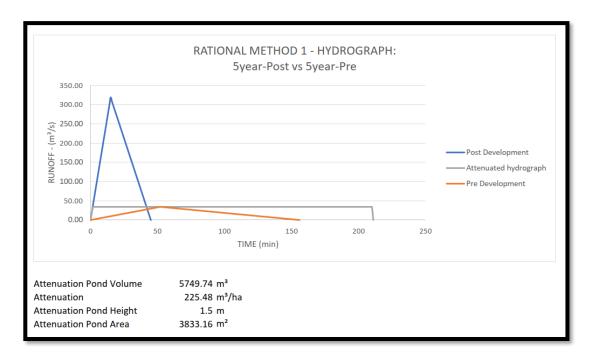


Figure 3: 5y-Pre vs 5y-Post Hydrograph

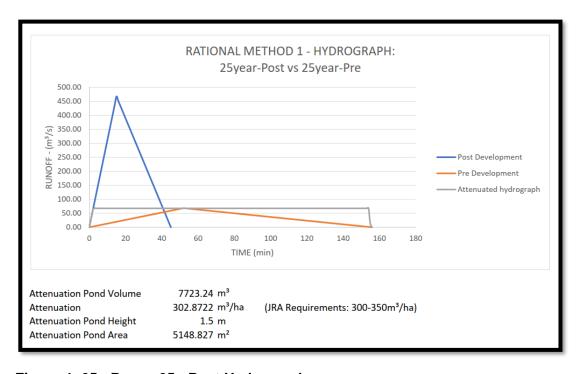


Figure 4: 25y-Pre vs 25y-Post Hydrograph

Table 9.3.1 Estimated stormwater attenuation volumes – Simplified Hydrograph

		Pre- development (m³)	Post- development (m³)	Attenuation Pond Volume (m³)
Lanseria	1:5 year	2667.6	7168.5	4500.9
Extension 1 Southern Precinct	1:25 year	5335.2	10 503	5167.8

<sup>\*</sup> Simplified Hydrograph - 3TC

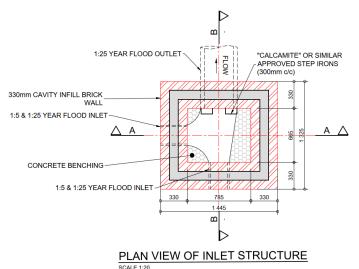
The estimated design summary is included in **Annexure I.** 

The designs of the attenuation facilities were done to size the pond for optimum storage volume. The proposed positions of the attenuation facilities are located across the application site and are indicated on the layout in **Annexure H**. The accumulated volume of all the attenuation facilities on the size exceeds the required attenuation volume as indicated in *Table 9.3.1*.

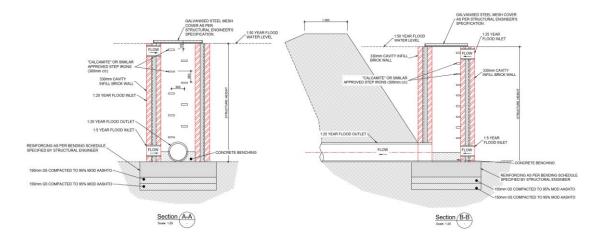
The proposed attenuation facilities will have a concrete-lined channel directing stormwater to the controlled outlet structure to prevent standing water.

#### 9.3.2 ATTENUATION POND OUTLET CONTROL STRUCTURE

The post-development stormwater will be managed and discharged in accordance with pre-development conditions through the attenuation pond outlet structure. This system will effectively control flood events with return periods of 1 in 5 years, and 1 in 25 years and a pond overflow weir structure will accommodate the 1 in 100 years storm event.



#### LANSERIA AIRPORT EXTENSION 1 – SOUTHERN PRECINCT



#### 10 CONCLUSIONS

It can be concluded that:

- The stormwater runoff would increase due to the proposed development.
- Upstream stormwater of Lanseria X75 needs to be managed through Lanseria X1 southern precinct.
- The stormwater runoff for the 1:100 year storm event can be discharged from the site by means of internal road surface flow that finally discharges into the natural watercourse on the eastern side to ensure no flooding occurs on site.
- The quantity and rate of stormwater runoff from the site will be buffered and controlled as per the requirements of the Johannesburg Roads Agency by means of attenuation facilities.
- Lanseria Extension 1 southern precinct attenuation pond outlet will discharge in Lanseria Extension 11 stormwater connection point.

#### 11 DEVELOPMENT ACCESS

The proposed developments will be located within the existing township of Lanseria Airport Extension 1, which currently has established access. An additional access point to the southern precinct of Lanseria Airport Extension 1 will be provided through the approved Lanseria Extension 11 township. All necessary servitudes for access and services will be registered in accordance with the provisions set out in the Service Level Agreement. Refer to **Annexure J** for the existing and proposed access locations.

#### 12 RECOMMENDATION

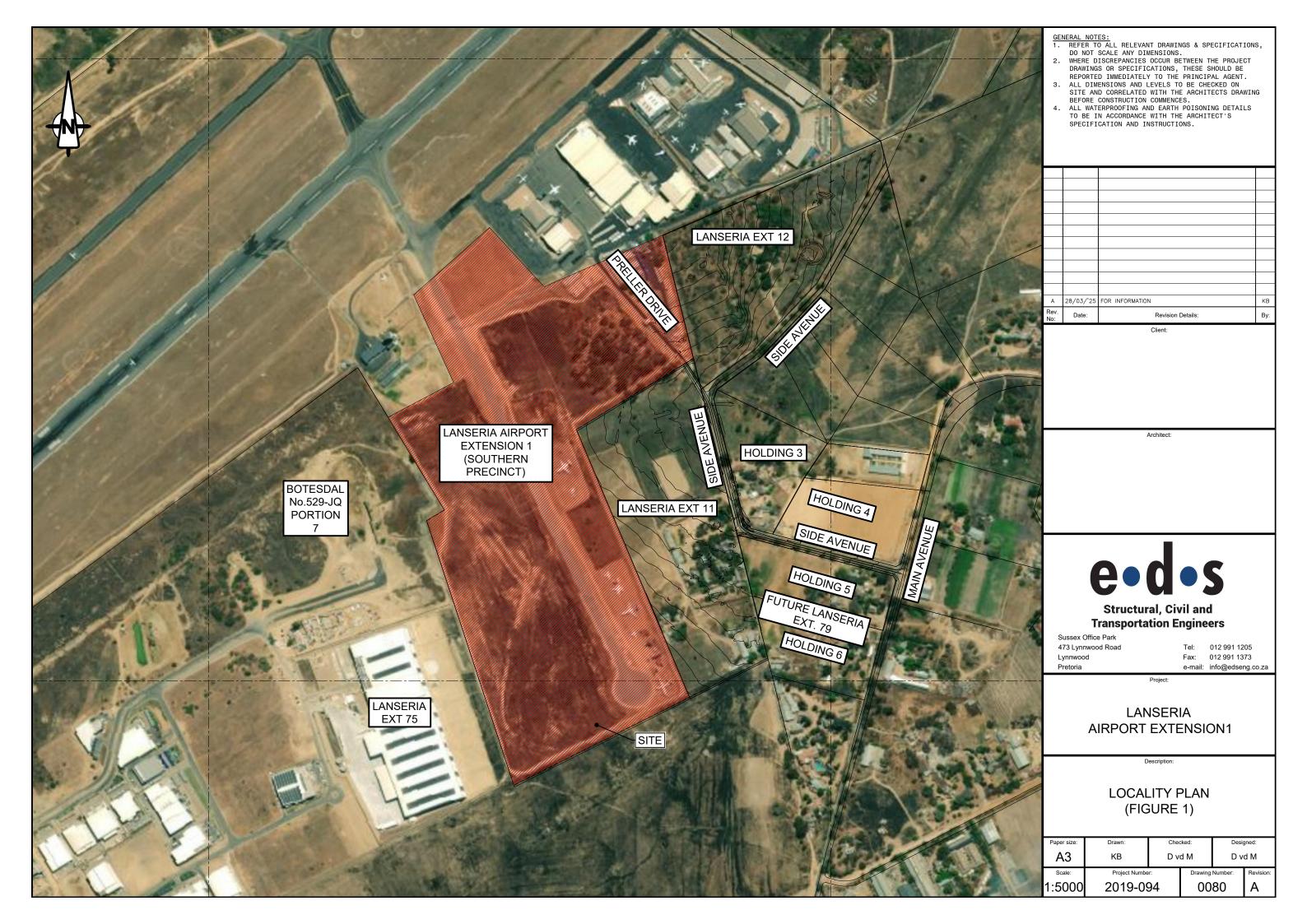
It is recommended that the Stormwater Management Report for the township of Lanseria Extension 1 (Southern Precinct) be supported by the Johannesburg Roads Agency (JRA) as the quantity and the rate of stormwater runoff from the site can be controlled as per the requirements of JRA.

F.H.B van Eyk Pr. Eng. (20160826)

#### **Annexures:**

- Annexure A: Site Location Map
- Annexure B: Proposed Township Layout
- Annexure C: Existing Land-Use Rights
- Annexure D: Existing JRA Stormwater Information
- Annexure E: Existing Site Stormwater Management
- Annexure F: Approved SWMR
- Annexure G: Flood line study
- Annexure H: Stormwater Management Layout Drawing
- Annexure I: Stormwater Calculations
- Annexure J: Development Access

## **ANNEXURE A: SITE LOCATION MAP**



## **ANNEXURE B: PROPOSED TOWNSHIP LAYOUT**



## **ANNEXURE C: EXISTING LAND-USE RIGHTS**

## DEVELOPMENT PLANNING LEGAL ADMINISTRATION

## Memo

TO

GIS

J THOMAS

**VALUATION SERVICES** 

OUR REF:

03-17300

DATE

12 March 2019

#### ERF 183 Lanseria Airport Ext.1

Attached, please find copies of approved Map 3 documents as well as the Notice to the Gazette for proclamation, for your records.

DEPUTY DIRECTOR: LEGAL ADMINISTRATION DEVELOPMENT PLANNING

Phaswana Mulalo

TEL: 407-7119 Z FAX: 339-1707

#### PLAASLIKE OWERHEID KENNISGEWING 415 VAN 2019

#### STAD VAN TSHWANE METROPOLITAANSE MUNISIPALITEIT

KENNISGEWING VAN 'N AANSOEK VIR DIE OPHEFFING VAN BEPERKENDE TITELVOORWAARDES IN DIE TITELAKTE INGEVOLGE ARTIKEL 16(2) VAN DIE STAD TSHWANE GRONDGEBRUIKBESTUURSBYWET, 2016

Ons, The Town Planning Hub cc, synde die gemagtigde agent/aansoeker van **Gedeelte 20 van die plaas Hartebeestfontein 484-JR**, gee hiermee ingevolge Artikel 16(1)(f) van die Stad Tshwane Grondgebruikbestuur Verordening, 2016 kennis dat ons by die Stad van Tshwane Metropolitaanse Munisipaliteit aansoek gedoen het vir die verwydering van sekere voorwaardes soos vervat in die Titel Akte in terme van Artikel 16(2) van die Stad Tshwane Grondgebruikbestuur Verordening, 2016 van die bogenoemde eiendom. Die eiendom is geleë suid van die R495; oos van die stad Rayton op pad na Ekangala in die ooste.

Die aansoek is vir die opheffing van voorwaardes (b)(i), (b)(ii), (b)(iii) in Titelakte T44692/2005 van die eiendom. Die bedoeling van die eienaar is om die bestaande trou venue te wettig deur middel van 'n toestemmingsgebruik aansoek. Daar is egter beperkende voorwaardes vervat in die Titelakte, wat verwyder moet word.

Enige besware en/of kommentare wat duidelik die gronde van die beswaar, asook die persoon(ne) se volle kontakbesonderhede, waar sonder die Munisipaliteit nie met die persoon(ne) kan korrespondeer nie, moet binne 'n tydperk van 28 dae vanaf 6 Maart 2019, skriftelik by of tot die Strategiese Uitvoerende Direkteur: Stadsbeplanning en Ontwikkeling, ingedien of gerig word by Posbus 3242, Pretoria, 0001, of na CityP\_Registration@tshwane.gov.za tot 3 April 2019.

Volledige besonderhede en planne (as daar is) kan gedurende gewone kantoorure geinspekteer word by die Munisipale kantore soos hieronder uiteengesit, vir 'n tydperk van 28 dae vanaf die datum van eerste publikasie van die kennisgewing in die Provinsiale Koerant, Beeld en Citizen koerante.

Adres van Munisipale Kantore: Munisipale Kantore, Isivuno House, Kamer LG004, 143 Lilian Ngoyistraat, Pretoria.

Sluitingsdatum vir enige besware en/of kommentaar: 3 April 2019

Adres van agent: The Town Planning Hub cc; Posbus 11437, Silver Lakes, 0054; 98 Pony Straat, Tijgervallei Kantoor Park,

Silver Lakes, Pretoria. Tel: (012) 809 2229 Faks: (012) 809 2090. Ref: TPH18281

Datums waarop die advertensie geplaas word: 6 en 13 Maart 2019 Verwysing nr: CPD484-JR/1004/00020 Item nr: 29894

6-13

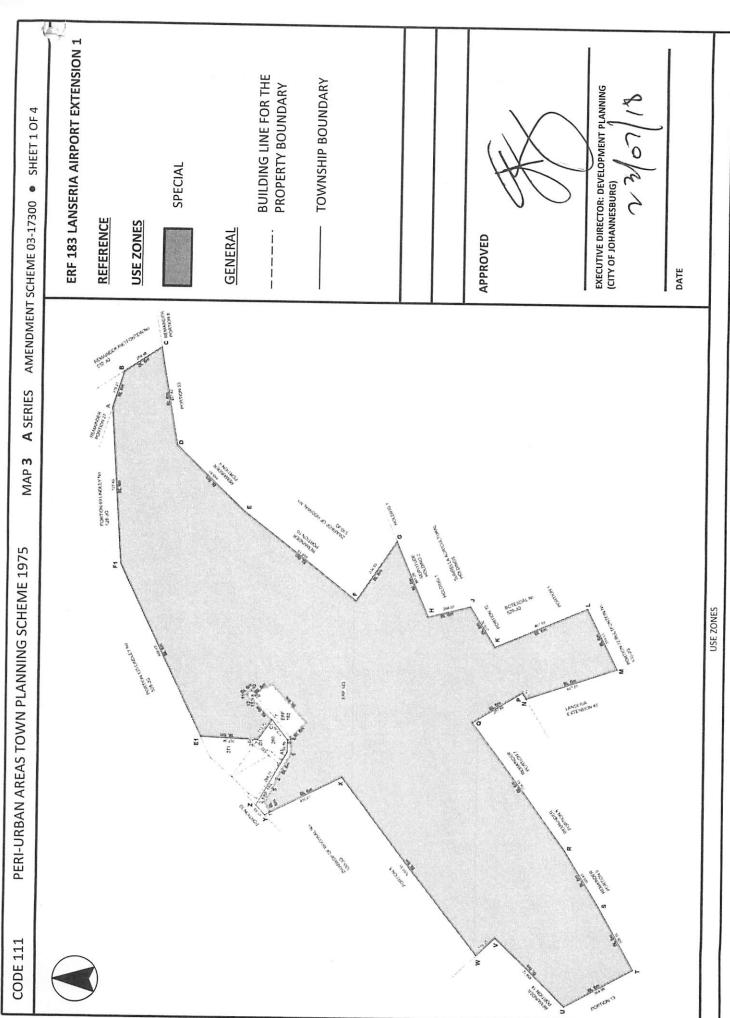
#### **LOCAL AUTHORITY NOTICE 416 OF 2019**

#### **AMENDMENT SCHEME 03-17300**

Notice is hereby given in terms of Section 22(4) of the City of Johannesburg Municipal Planning By-Law, 2016, that the City of Johannesburg Metropolitan Municipality has approved the amendment of the Peri-Urban Areas Town Planning Scheme, 1975, by the rezoning of Erf 183 Lanseria Airport Extension 1 from "Special" to "Special", subject to certain conditions as indicated in the approved application, which Amendment Scheme will be known as Amendment Scheme 03-17300 Amendment Scheme 03-17300 will come into operation on the date of publication hereof.

The Amendment Scheme is filed with the Executive Director: Development Planning, 158 Civic Boulevard, Metropolitan Centre, A Block, 8<sup>th</sup> Floor, Braamfontein 2017 and is open for inspection at all reasonable times.

Hector Bheki Makhubo
Deputy Director: Legal Administration
City of Johannesburg Metropolitan Municipality /
Notice No 187/2019



DATE

**Building Lines:** 

As Per Scheme.

Provided that the building lines may be relaxed on

approval of a Site Development Plan.

#### General

- 1. A Site Development Plan, compiled to a scale of 1:500 or such other scale as may be approved by the local authority shall be submitted for approval to the local authority prior to submission of any building plans. No buildings shall be erected on the erf until such Site Development Plan has been approved by the local authority, and the entire development of the erf shall be in accordance with the approved site development plan.
- 2. Access to and ingress from the erven shall be to the satisfaction of the relevant authorities.
- 3. All comments from the City's Municipal Owned Entities (MOE's) to be adhered to, to the satisfaction of the council.
- 4. All conditions stipulated by the Civil Aviation Authority to be adhered to, to the satisfaction of the council.

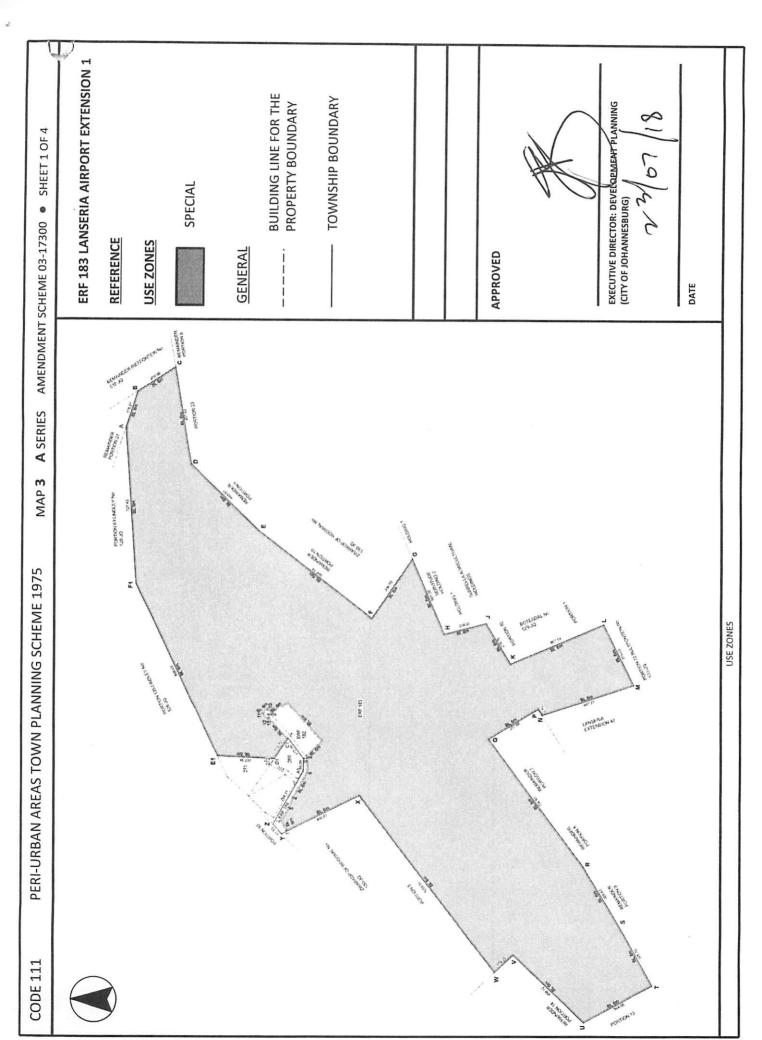
ERF 183 LANSERIA AIRPORT EXTENSION 1

**APPROVED** 

EXECUTIVE DIRECTOR: DEVELOPMENT PLANNING

(CITY OF JOHANNESBURG)

DATE



ERF 183 LANSERIA AIRPORT EXTENSION 1 BUILDING LINE FOR THE PROPERTY BOUNDARY (6 m AS PER SCHEME) **TOWNSHIP BOUNDARY** EXECUTIVE DIRECTOR: DEVELOPMENT PLANNING (CITY OF JOHANNESBURG) AMENDMENT SCHEME 03-17300 ● SHEET 2 OF 4 0 REFERENCE GENERAL **APPROVED** DATE **A** SERIES MAP 3 FUKTION 64 LEDLEY NO 128-LD DENSITY ZONES AND HEIGHT ZONES PERI-URBAN AREAS TOWN PLANNING SCHEME 1975 **CODE 111 2CALE 1:12500** 

**Building Lines:** 

As Per Scheme.

Provided that the building lines may be relaxed on

approval of a Site Development Plan.

#### General

- 1. A Site Development Plan, compiled to a scale of 1:500 or such other scale as may be approved by the local authority shall be submitted for approval to the local authority prior to submission of any building plans. No buildings shall be erected on the erf until such Site Development Plan has been approved by the local authority, and the entire development of the erf shall be in accordance with the approved site development plan.
- 2. Access to and ingress from the erven shall be to the satisfaction of the relevant authorities.
- 3. All comments from the City's Municipal Owned Entities (MOE's) to be adhered to, to the satisfaction of the council.
- 4. All conditions stipulated by the Civil Aviation Authority to be adhered to, to the satisfaction of the council.

**ERF 183 LANSERIA AIRPORT EXTENSION 1** 

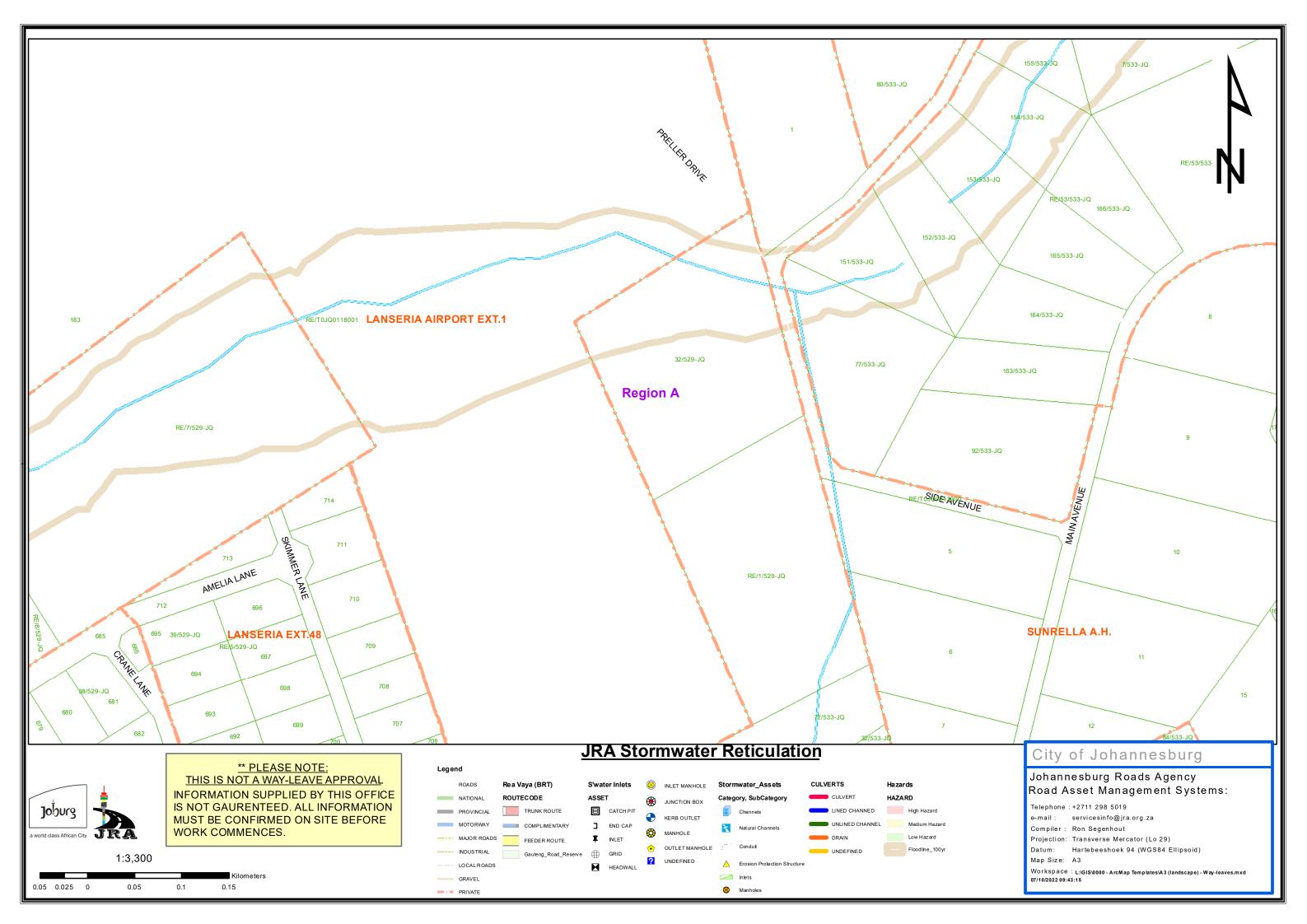
**APPROVED** 

EXECUTIVE DIRECTOR: DEVELOPMENT PLANNING

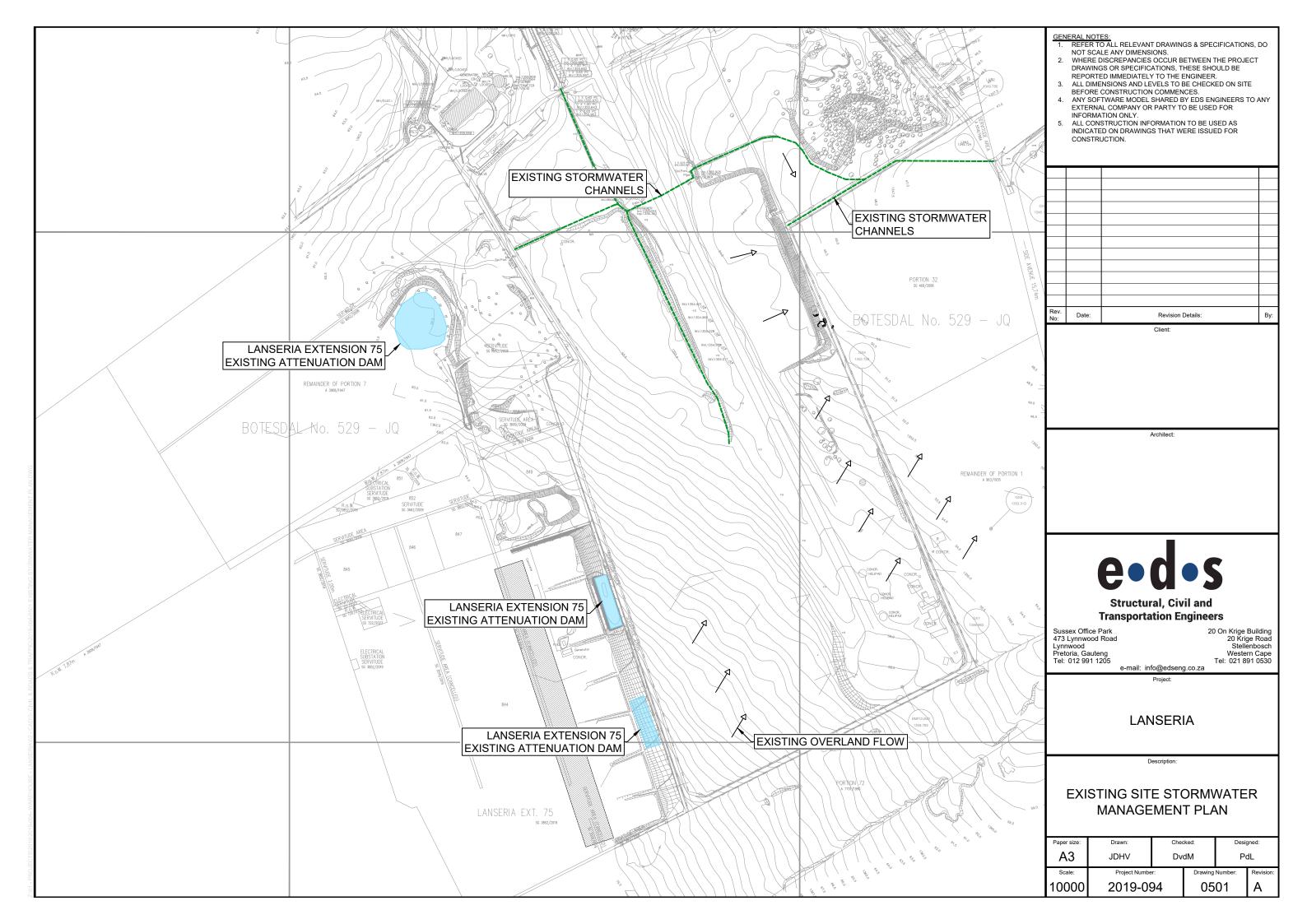
(CITY OF JOHANNESBURG)

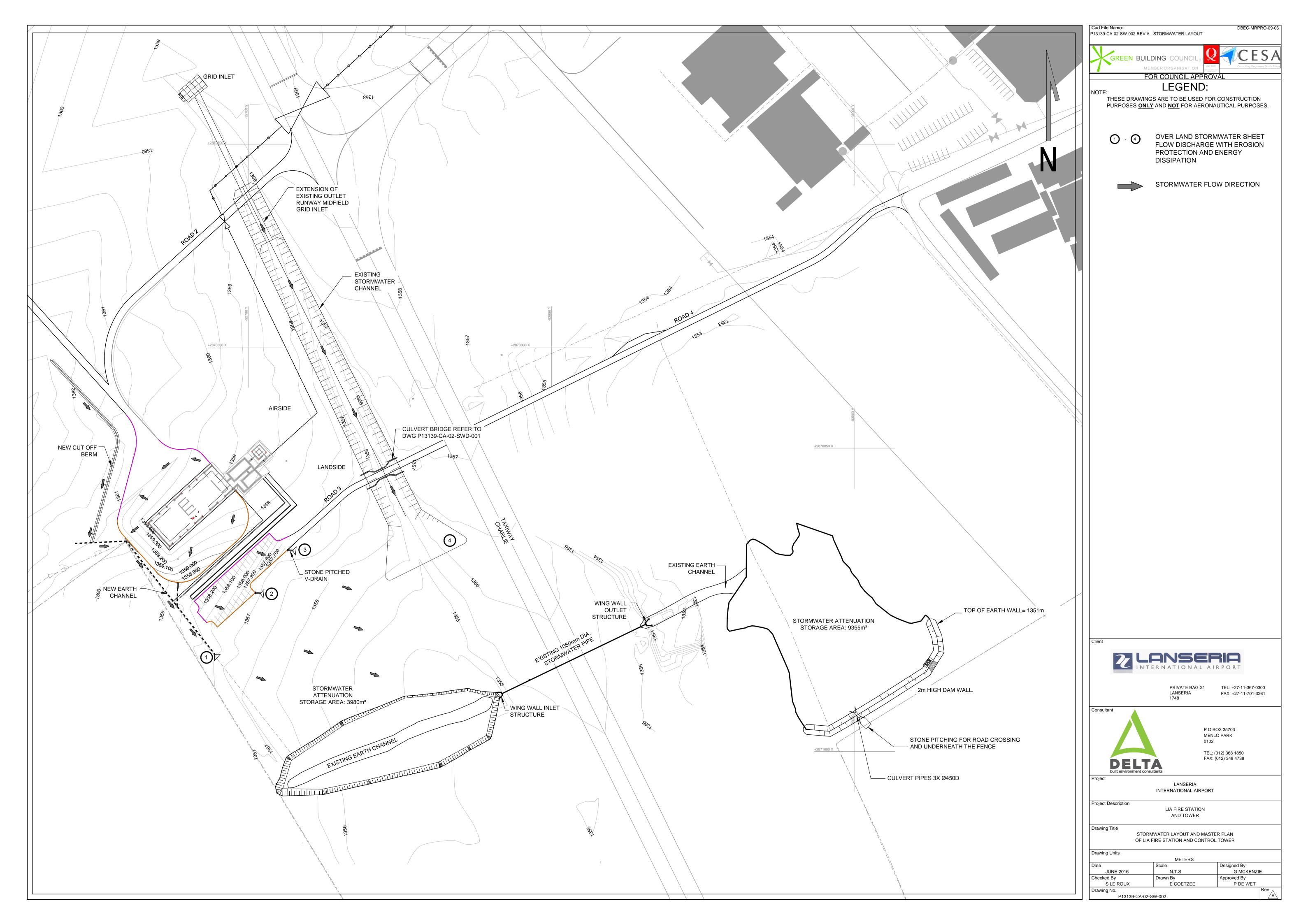
DATE

# ANNEXURE D: EXISTING JRA STORMWATER INFORMATION



# ANNEXURE E: EXISTING SITE STORMWATER MANAGEMENT





## **ANNEXURE F: APPROVED SWMR**



Plan your journey Use alternative routes Consider using public transport



Alternative routes on www.jra.org.za



#### Good day

Herewith a letter for Lanseria extension 1.

Regards Vincent Dev control

----Original Message----

From: administrator@jra.org.za [mailto:administrator@jra.org.za]

Sent: 31 August 2016 11:01 AM

To: Vincent Mphilo

Subject: Message from "RNP0026734702FE"

This E-mail was sent from "RNP0026734702FE" (Aficio MP 2352).

Scan Date: 31.08.2016 11:01:19 (+0200) Oueries to: administrator@jra.org.za

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City of Johannesburg Johannesburg Roads Agency

66 Pixley Ka Isaka Seme Street Cnr. Rahima Moosa Street Johannesburg

P/Bag X70 Braamfontein South Africa 2017

Ref: 17/8/L47-1

N. Chinyowa

Tel +27(0) 11 298 5000 Fax +27(0) 11 298 5178 www.jra.org.za www.joburg.org.za

Tel: (011) 298-5043

Fax: (011) 298-5066

Delta Built Environment Consultants PO Box 35703 Menlo Park 0102

Date: 25 August 2016

Email: piet.dewet@deltabec.com

Attention: Mr Piet de Wet

Sir,

STORMWATER DRAINAGE MASTER PLAN FOR LANSERIA AIRPORT EXTENSION 1

With regards to your revised Stormwater Master Plan received on 18 August 2016 for Lanseria Airport Extension 1, please receive the following comments:

1. The stormwater management plan is acceptable.

- 2. The total stormwater attenuation for the township as per the report is 36 432m3. This falls short of the JRA requirement of at least 300m³/ha which would equate to 79 392m³. However given that the approved FAR for erf 183 is 0.2 the designed attenuation is adequate. Should the FAR be increased on any stand in the township, the JRA will require stormwater attenuation at the acceptable rate of 300m<sup>3</sup>/ha to 350m<sup>3</sup>/ha.
- 3. Your internal stormwater management system will remain private.
- 4. All municipal servitudes should be protected.
- 5. The township layout should conform to the JRA comments on the Traffic Impact Study.
- 6. All upgrades and amendments to the roads network around the site as per the JRA comments should be implemented.
- 7. Any portion of the airport that is to be developed should conform to this Stormwater Master Plan, and should clearly set out the infrastructure to be installed.
- 8. Prior to construction, the Detailed Design showing the site layout, stormwater collection point(s), stormwater attenuation pond details, outlet details and erosion control, invert levels and longsections, should be submitted and approved.

#### Please note:

All road upgrades to be undertaken by the developer or his representatives, the cost thereof, will not be refunded back to the developer by the Johannesburg Roads Agency (JRA) or the City of Johannesburg (CoJ) unless these upgrades were discussed and agreed upon in writing by both parties

Chairman: J Mancho,
Executive Directors: Dr. S Phillips - Managing Director, G Mbatha CA(SA) - Chief Financial Officer
Non-Executive Directors: P Govender, J Maina, A Torres, N Msezane, E Ngomana, L Mashamaite, L Nxumalo, H Mashele.

upfront, before any construction commences. The mere fact that the detail design drawings or Traffic Impact Studies have been approved, does not bind the JRA or the CoJ to any agreement.

It should also be noted that if any upgrades are undertaken by the developer to any roads or stormwater on behalf of CoJ or the JRA, the developer will be entitled to an off-set against their external engineering services contributions as per section 49(4) of SPLUMA, provided these services are required to be upgraded to resolve background capacity problems, and not as a result of his/her impact of the development. These upgrades are to be discussed with the officials of the JRA and agreement in writing is to be obtained from the JRA to the off-set of such contributions, before any construction commences on site.

If the amount for the upgrade/construction exceeds the contributions payable, the balance thereof will not be refunded to the developer and the construction is then carried out at the developers own cost.

Yours faithfully

pp Manager: Development Control

nc/nc

## **ANNEXURE G: FLOOD LINE STUDY**

# **Lanseria Ext 11 Development Floodline study**

Report Prepared for: EDS Engineers

Report Prepared by M Braune , Pr Eng March 2023

# 1:100 Year Floodline study: Lanseria Ext 11 Development

#### **EDS Engineers**



#### **Bio Engineering Solutions (Pty) Ltd**



440 Kings highway

Lynnwood

Pretoria

mattbraune7@gmail.com

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#### 1 Introduction and Scope of Report

We refer to your request for a floodline study for a development adjacent the Lanseria airport. It is planned to develop a site for usage as a storage facility. There is also a legal requirement in terms of the National Water Act of 1998(Act No 36) that a 1:100-year floodline study be carried out along any watercourse and/ or depression in the vicinity of an existing and planned development. The Locality of the planned development on **Figure 1-1** below

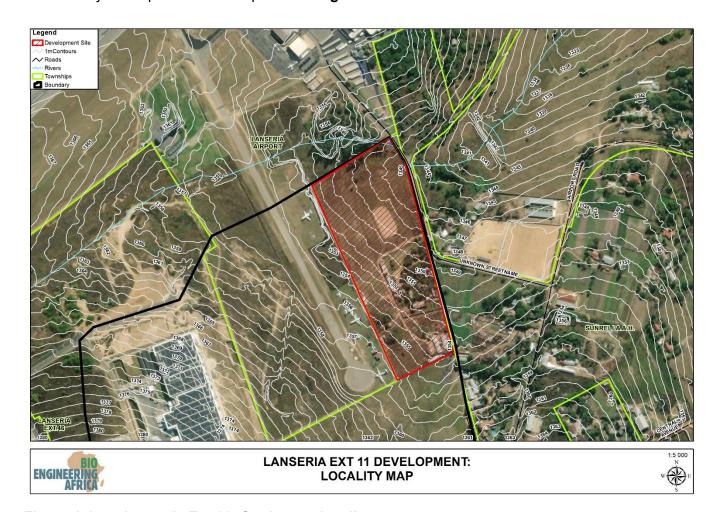


Figure 1-1: Lanseria Ext 11 :Study area locality

#### 2 Description of Study Area and watercourse

The study area is situated within a developing area to the east of the existing Lanseria airport within the city of Johannesburg municipal area. There is a upstream natural watercourse which drains towards the development site. The water course continues over the site and is drained underneath an access road via a culvert structure. Typical details of the watercourse and site are shown in **Figure 2-1** below.

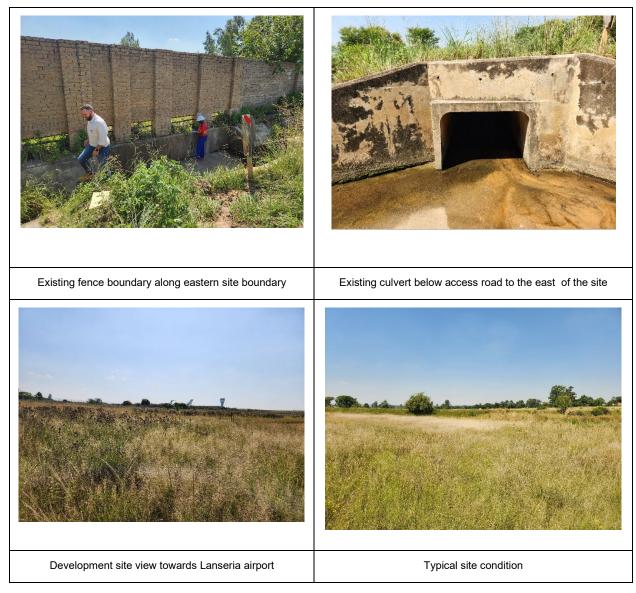


Figure 2-1: Typical details of the site and watercourse

#### 3 Existing Topographical Details

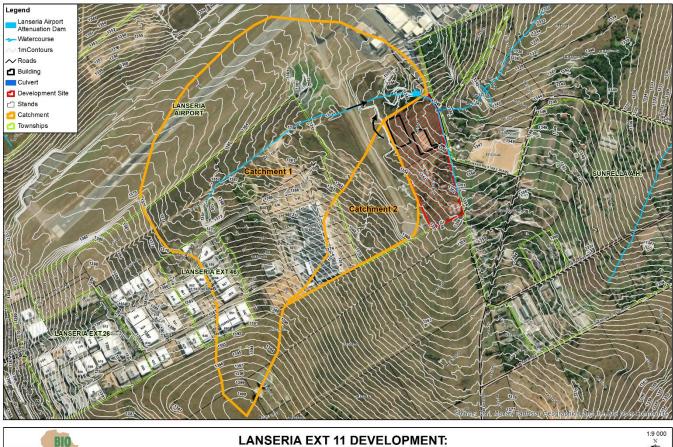
The general topography was determined using the latest available contour information from the city of Johannesburg . Additional field measurements of the control structures were taken during a site visit and a local detailed survey was forward by EDS Engineers

#### 3.1 Site Visit

A site visit of the study area was conducted to determine any changes within the surrounding area as well as along the watercourse and the development site. From the site visit it was determined that the existing contour information is accurate and reflects the existing topography.

#### Flood Hydrology 4

A hydrological study was carried out based on the current catchment conditions using the UPD model. The peak flow rates were determined by employing both the RATIONAL and ALTERNATIVE RATIONAL methods. The catchment areas and boundaries are shown on Figure **4-1** below for the main watercourse to the north of the development site . The catchment measures 0.8 km<sup>2</sup>.



ENGINEER

**CATCHMENT BOUNDARY** 



Figure 4-1: **Development site and catchment boundary** 

#### 4.1 Rainfall determination

An important input parameter to the hydrological model is the expected storm rainfall.

The rainfall records were obtained from the South African Weather Services (SAWS). The rainfall station 0475661 was used for this purpose . The rainfall station has a MAP of 646 mm . A summary of the determined 24-hr storm rainfall is given in Table 4.1 below.

Table 4-1: Storm rainfall

Return period ( yrs)	Storm rainfall ( mm)
1:50	131

1:100	152

A summary of the peak flows estimated by the various methods is given in Table 4-2 below. Further details of the model parameters are given in **Appendix A**.

#### 4.2 Land-use

Existing and future planned land-use use was obtained from the spatial development framework and is shown in Figure 4-2 below

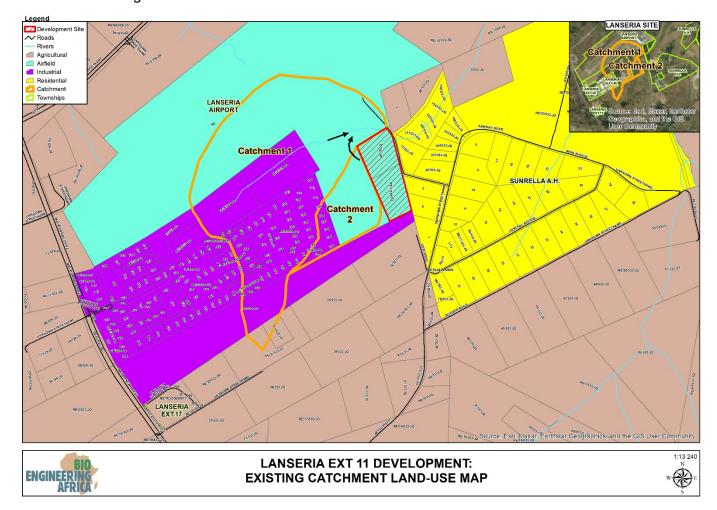


Figure 4-2: Future land-use coverage

It is seen from the above Figure that the catchment coverage consist of a mixture of commercial /industrial and residential development. Based on the above a RATIONAL weighted average coefficient C = 0.70 was obtained.

#### 4.3 Regional geology

A further input parameter to the hydrological model is the current soil condition . It is seen from the regional geological map shown on Figure 4-3 that the catchment is within the Granite Gneiss formation .This gives a shallow soil coverage with underlaying rocks and a medium-high runoff potential .



Figure 4-3: Regional Geology

#### 4.4 Peak flow determination

Taking the above catchment as well as model input parameters into account the UPD software was used to determine relevant peak flow rates. A summary of the expected peak flow rates is given below in Table 4-2.

Table 4-2: Summary of peak flows

Method	1:50 year peak flow ( m³/s )	1:100 year peak flow ( m³/s )		
Rational Method	20,0	26,0		
Alternative Rational	24,4	28,5		
Adopted	24,4	28,5		

Based on recommendations from the Urban Drainage Manual ( 6<sup>th</sup> Edition ) as well as the UPD program literature the method selected as being most accurate is the <u>Alternative Rational</u>.

#### 5 HECRAS Model Compilation

This program employs detailed channel morphology as well as site-specific hydrological data combined to perform one-dimensional hydraulic calculations for a river network. The HECRAS model employs standard backwater techniques to compute the high-water level for various steady flow conditions, considering structures and controls across a watercourse. A HECRAS model was set up for the following two conditions:

- i. Existing natural watercourse;
- ii. Planned new culvert drainage system

The HECRAS model main parameters are summarised in **Table 5-1** below.

Table 5-3: HECRAS Model Main Parameters

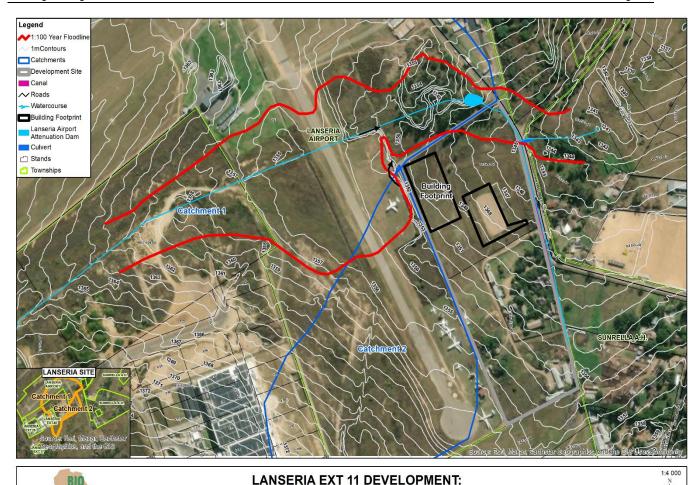
Parameter Average Value/Selection		Reason							
Existing Natural watercourse									
Manning 'n'	0.040 (main flow channel)	Defined natural watercourse							
	0.030 (floodplains )	Light bush and grassed area							
Boundary conditions	Critical flow depth	Inlet and outlet structures							
Flow regime	Mixed flow	Slope and cross section changes requiring super and sub-critical flow regimes							
	New culvert drainag	e system							
Manning 'n'	0,012	Concrete culvert							
Boundary conditions Critical flow depth		Inlet and outlet structures							

#### 6 Findings of the Floodline Study

The 50/100 year flood lines have been determined based on the two conditions listed above .

#### 6.1 Existing watercourse conditions

The determined floodlines for a 1:50 and 1:100-year events for the current natural watercourse are shown in **Figure 6-1** below.



50/100 YEAR FLOODLINE & EXISTING WATERCOURSE

Figure 6-1: 50/100 year floodlines (Existing watercourse condition)

The following is observed from the above figure:

- i. A wide area is covered by the floodlines under current watercourse conditions due to the flat topography upstream of the site;
- ii. Only a small portion of the site at the north-eastern corner is affected under current development conditions;
- iii. The flow depth and velocity is low due to the wide flood plain hence giving no erosion problems.

#### 6.2 Planned future drainage system condition

As part of the development site it is planned to construct a underground culvert drainage system . The culvert will start at the western boundary of the site and drain all the stormwater along the northern boundary of the site as well as the paved area upstream of the site discharging into the downstream natural watercourse. The Culvert drainage system has been sized to handle up to a 1:100 year flood peak without causing any flooding nor damage to the surrounding area and new

development . A **3,0 m wide by 2,4 m high** portal culvert is required . The culvert inlet works should be designed in relation to the proposed concept design given in Figure 6-2 below .

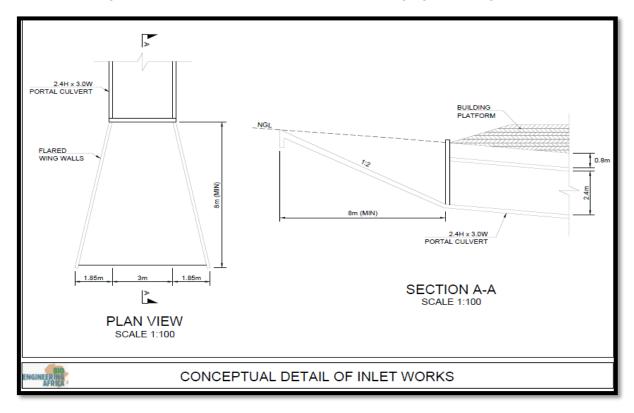


Figure 6-2: Culvert inlet works concept design

It is also planned to implement a side flow channel at the toe of the backfilled building terrace to intercept and drain all overland upstream runoff into the culvert system. The channel should be either lined with concrete or grouted stone pitching. The minimum size would need to be:

> Bottom width :3,0 m

> Depth( min): 1,5 m

➤ Side slope 1:1,5

The HECRAS model was now again set up with the planned culvert drainage system .The modified floodline is shown on Figure 6-3 below .

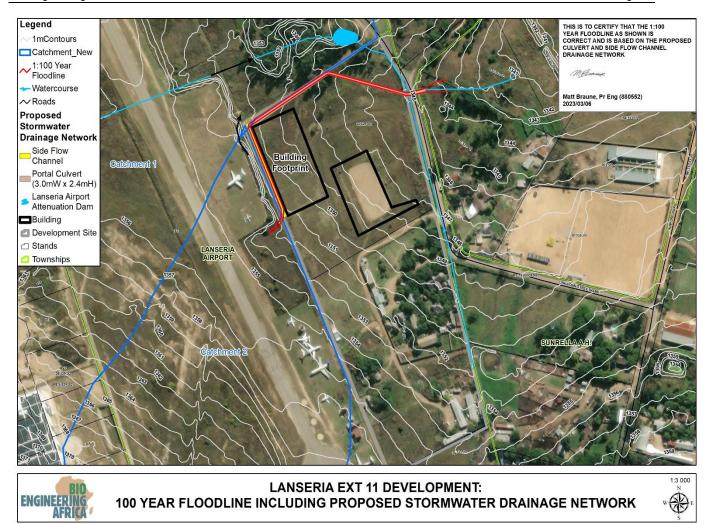


Figure 6-3: 50/100 year floodlines (with planned culvert drainage system )

The following is observed from Figure 6-3 above:

- i. The revised floodline will no longer have an impact of the development site;
- ii. The floodline will be contained along the western boundary of the site by a side flow channel which then drains into the planned underground portal culvert along the north site boundary.

#### 7 Conclusions

The following is concluded from the floodline study:

 i. The 1:50 and 1:100-year flood floodline encroaches only a small portion of the north-eastern corner of the development site under current water course conditions;

- ii. If a side flow channel and a culvert drainage system is implemented the 100 year floodline will no longer encroach nor have any impact on the development site;
- iii. A 3,0m wide x 2,4 m high portal culvert is required to handle up to a 1:100 year flood event
- iv. A side flow channel (bottom width 3m x depth 1,5 m and side slope of 1:1,5) would be required to drain all upstream flow into the culvert system

#### 8 Recommendations

The following is recommended:

- i. The floodlines be used for township planning;
- ii. The planned stormwater drainage system be constructed as part of the new development

#### Prepared by

M. Braune, Pr Eng (880552)

Meliner

## Appendix A:

**UPD** model results

#### Utility Programs for Drainage Flood calculations



Sinotech

Project name: LANSERIA DEVELOPMENT

Analysed by: J. AKURA

LANSERIA STREAM Name of river:

Description of site: DEVELOPMENT SITE NEXT TO LANSERIA AIRPORT
Filename: C:\Users\User\OneDrive\Projects\02 - Projects\0699-EDS Lanseria Floodlines\
04 Design\UPD\FUTURE CONDITIONS\FUTURE CONDITIONS (1.15 RAINFALL)- LANSERIA DE\

3 March 2023

Printed: 6 March 2023

Flood Frequency Analysis: Alternative Rational Method

- LANSERIA DEVELOPMENT
- J. ANDRA
- J. ANDRA
- LANSERIA STREAM
- DEVELOPMENT SITE NEXT TO LANSERIA AIRPORT
- 2023/03/03
- 0.72 km²
- 40.0 %
- 1.463 km
- Defined water course
- 46.4 m Analysed by Name of river Description of site Date

Date
Area of catchment
Dolomitic area
Length of longest watercourse
Flow of water
Height difference along 10-85 slope
Area distribution

= 46.4 m = Rural: 42 %, Urhan: 57 %, Lakes: 1 %

Beight difference annual Read distribution

Catchment description - Urban area (%)

Lawns

Sandy, flat (<2%) 1 Houses 2 City centre Sandy, steep (>7%) 0 Flats 1 Schumban

Heavy soil, flat (<2%) 0 Light industry 36 Streets

Heavy soil, steep (>7%) 0 Heavy industry 0 Maximum flood

Catchment description - Rural area (%)

Surface slopes Permeability Vegetation

Lakes and pans 1 Veny permeable 0 Thick bush 5 flat area 99 Permeable 2 Light bush 5 flat area 99 Permeable 10 Grasslands

Steep areas 0 Impermeable 00 Bare

Days on which thunder was heard 60 days/year

Vegetation
Thick bush & forests 0
Light bush & cultivated land 0
Grasslands 10
Bare 9

Awarage slope Time of concentration Pun-off factor Fural - Cl - 0.04229 m/m - 10.0 min - 0.599 - 0.751 - 0.000

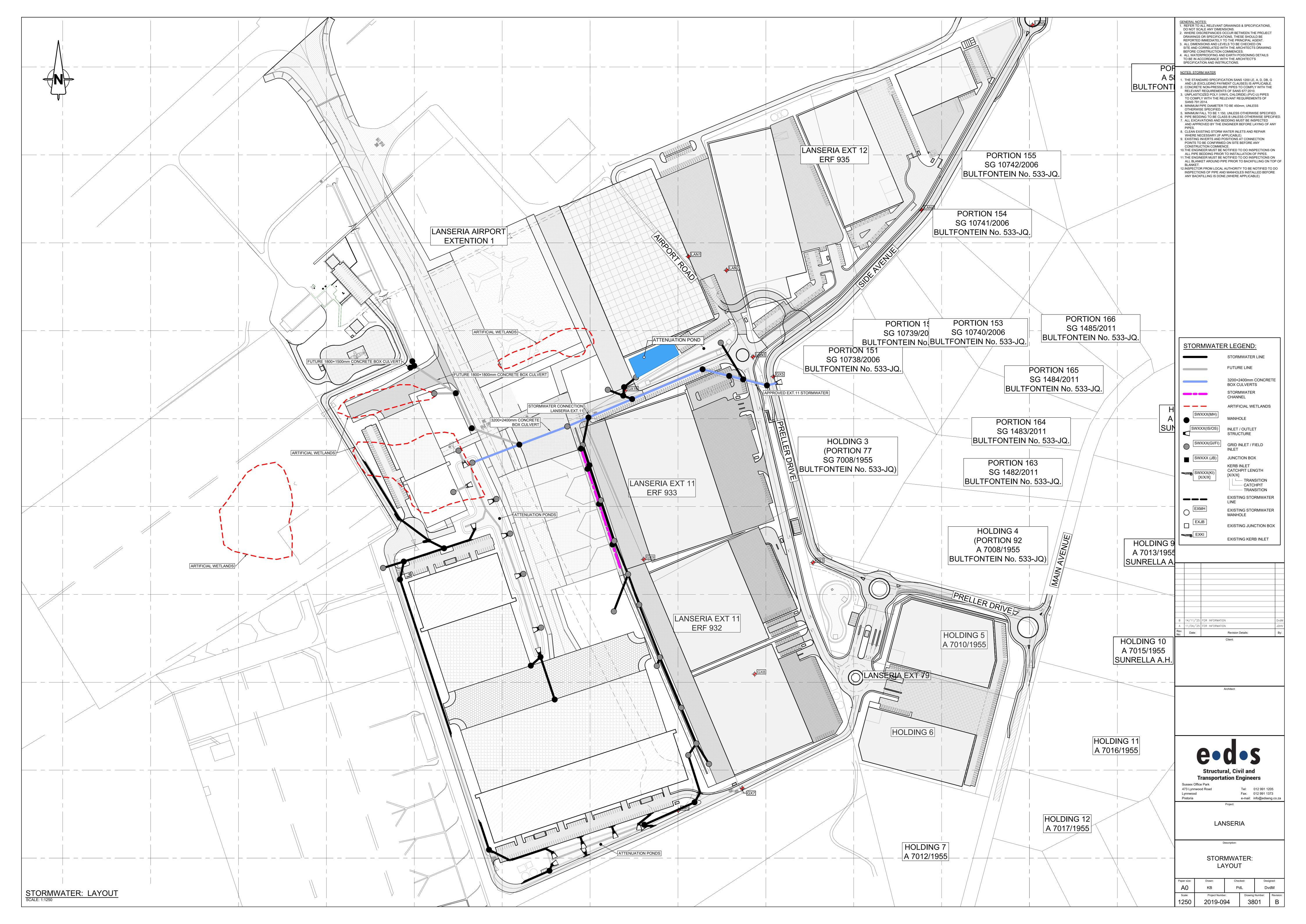
Combined - C = 0.599 Bural run-off coefficient Cl above includes dolomitic factors

Return period (years)	Time of concentration (hours)	Point rainfall (mm)	(4)	intensity (mm/h)	Factor	Runoff coefficient (%)	flow (m'/s)
1:2	0.30	10.10	100.0	60.50	0.75	55.6	6.730
1:5	0.30	30.67	100.0	102.05	0.00	56.5	11.53
1:10	0.30	40.12	100.0	133.49	0.85	57.3	15.31
1:20	0.30	49.56	100.0	164.93	0.90	50.2	19.20
1:50	0.30	62.05	100.0	206.49	0.95	59.0	24.39
:100	0.30	71.50	100.0	237.93	1.00	59.9	20.50
1:200	0.30	80.95	100.0	269.37	1.00	59.9	32.27

Run-off coefficient percentage includes adjustment saturation factors (Ft) for steep and impermeable catchments

Calculated using Utility Programs for Drainage 1.1.0

## ANNEXURE H: STORMWATER MANAGEMENT LAYOUT DRAWING



## **ANNEXURE I: STORMWATER CALCULATIONS**

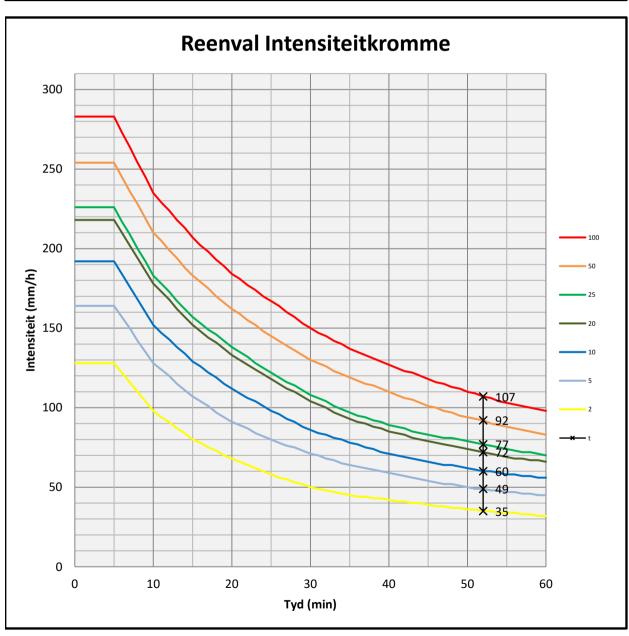
#### Stormwater - Standaard Intensiteitskrommes

Klas =	В
t =	52

Carolina, Cedara, Estcourt, Jan Smuts, Kokstad, Krugersdorp, Mafeking, Piet Retief, Potchefstroom, Pretoria, Roodeplaat, Rustenburg, Sheeprun, Towoomba

Tyd waarvoor die Intensiteit bepaal word (min)

Herhaalings Periode T	100	50	25	20	10	5	2
Tyd t (min)	52	52	52	52	52	52	52
Reenval Intensiteit I (mm/hr)	107	92	77	72	60	49	35



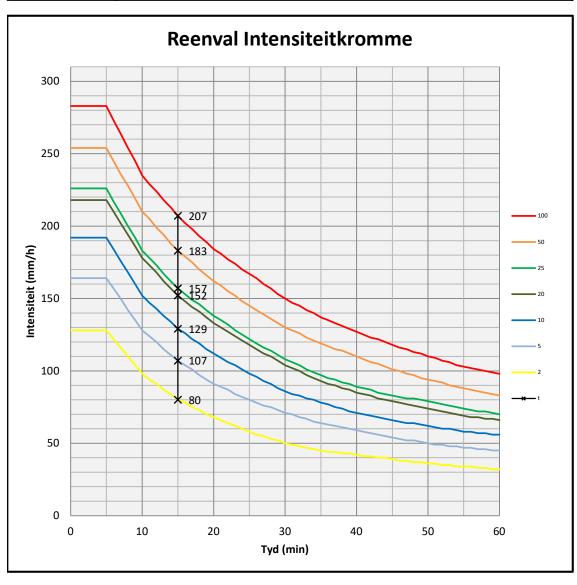
#### Stormwater - Standaard Intensiteitskrommes

Klas =	В
t =	15

Carolina, Cedara, Estcourt, Jan Smuts, Kokstad, Krugersdorp, Mafeking, Piet Retief, Potchefstroom, Pretoria, Roodeplaat, Rustenburg, Sheeprun, Towoomba

Tyd waarvoor die Intensiteit bepaal word (min)

Herhaalings Periode T	100	50	25	20	10	5	2
Tyd t (min)	15	15	15	15	15	15	15
Reenval Intensiteit I (mm/hr)	207	183	157	152	129	107	80



ourse (L) (S <sub>m</sub> )		RATIONAL M   RAT	ALMSHI AM	RATIONAL MISTHOD (ALTISRNATIVE I)   RATIONAL CHARACTERISTICS	D (ALBIERNATIVE 1) Southern Precinct ARE Rural (a) 100% Rainfall region ARE Rural (a) 100% Rainfall region ARE Rural (a) 100% Rainfall (a)	A DISTRIBU	Date 23 Overland flow Lanseria TION FACTOR U N	25,03/2025	C <sub>2</sub>
Welands and pans Flat areas Flat areas Steep areas Total Fermeability	100% 100% 0% 0% 100%	0.03 0.08 0.16 0.26	0.08 0.08 0.08	Lawns Sandy, flat Sandy, stee Heavy soil, Heavy soil, Residentia	(<2%) sp (>7%) flat (<2%) steep (>7%)		70	0.1 0.2 0.17 0.35	
Very permeable Permeable Semi-permeable Impermeable Im	50% 50% 0% 100% 50% 50%	0.04 0.08 0.16 0.26 - - Factor 0.04 0.11		Houses Flats Industry Light Indust Heavy Indust Heavy Indust Heavy Indust City centre Suburban Streets	rrial			0.5 0.7 0.8 0.9 0.95	
Crasslands	50% 0% 100% CENTRATIO	0.21 0.28 - N(T <sub>c</sub> )	105 0	Streets Maximum flood Total (C <sub>2</sub> ) Notes:	ood		0%	0.95	000
Overland flow®  r 0.7  L 0.563  Sav 0.03374778  To 51.75963 min To 0.88266 hours	Defi	0.033 7211 5712	0.563 0.03374778 0.03374778 7211 min 5712 hours	$T_{\mathcal{C}} =$	If $T_{\rm c}$ < 0.25 hours, $c = 0.604 (\frac{rL}{\sqrt{S_{av}}})^{0.467}$	If $T_{\rm C}$ < 0.25 hours, use $T_{\rm C}$ = 0.25 hours $0.4 \left(\frac{rL}{\sqrt{S_{av}}}\right)^{0.467} \qquad T_{\rm C} = \left(\frac{0.8}{100}\right)^{0.467}$	Ise $T_C = 0.2$ : $T_C = 0.2$	6 hours. $\frac{0.87L^2}{(1000S_{av})}$	0,385
n-off coeffi			2 0.3	COEFFICIENT 5 0.3	10 0.3	<b>20</b>	<b>25</b>	<b>50</b>	<b>100</b>
Adjusted for dolomitic areas, $C_{1D}$ (= $C_1(1 - D_\infty) + C_1D_\infty(\Sigma(D_{bador} \times C_{S_\infty}))^{\oplus}$	s‰))⊕		0.3	0.3	0.3	0.3	0.3	0.3	0.3
Adjustment factor for initial saturation, $F_t^{\otimes}$	ation,		0.50	0.55	0.60	0.67	0.70	0.83	1.00
Adjusted run-off coefficient, C <sub>1T</sub> (= C <sub>1D</sub> x F <sub>i</sub> )			0.15	0.165	0.18	0.201	0.209	0.249	0.3
Coombined run-off coefficiret, $C_7$ (= $\alpha C_{17} + \beta C_2 + \gamma C_3$ )			0.15 <b>RA</b>	0.17	0.18	0.20	0.21	0.25	0.30
Return period (years), T			2	5	10	20	25	50	100
Point rainfall (mm), P <sub>T</sub> ®									
Point intensity (mm/hour), $P_{ff}$ (= $P_T/T_C$ )	$P_T/\Gamma_C$ )		35	49	60	72	77	92	107
Area reduction factor (%), ARF <sub>T</sub> ® Average intensity (mm/hour), I <sub>T</sub>			100%	100%	100%	100%	100%	100%	100%
Average intensity (mm/nour), $I_T$ (= $P_{IT} \times ARF_T$ )			35	49	60	72	77	92	107
Return period (years), T			2	5	10	20	25	50	100
Peak flow (m3/s), Q=CIA/3.6			0.37	0.57	0.77	1.03	1.14	1.62	2.27

Stormwater - Standaard Intensitetiskrommes  Carolina, Codara Estocurt, Jan Smuts, Kolstad, Krugensdorp, Mafeking, Peul Redel, Pulcinelinoon, Pretiona, Roodeplani, Rusiemburg, Smeeprun, Towoomba
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(min)	

Reenval Intensiteit I (mm/hr)	Herhaalings Periode T  Tyd t (min)  Teenval Intensiteit I (mm/hr)	
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92	52	50
77	52	25
72	52	20
60	10 52 60	
49	52 52	
35	52	2

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	Google

Dividit	Description of Catchment		RATION/ 2019-094	L METHO	RATIONAL METHOD (ALTERNATIVE 1) 019-094 Lanseria X1 Southern Precinct	ATIVE 1)				
Catchiment(A)	Calculated By		DvdM						25/03/2025	
Columnitation   Columnitati			E E	SICAL CH	ARACTERIS	TICS				
AREA DISTREBUTION   ACCIDING   Infinite area (D. D.)   AREA DISTREBUTION   ACCIDING   Information   Infinite area (D. D.)   Inf	Size of catchment (A)		0.255	m,	Choose type	OFTIOW		Defined wat	ercourse	biro
mile area (D <sub>2</sub> )	q		0.028419	n/m	Namamieg		DISTRIBU	TION FACTO	a-Johanne DRS	grug
Particular   Pa	Dolomite area (D <sub>%</sub> )		0%			Rural (a)			Urban (β)	
Reside   Recipro   Recip	itation (M	)ø		nm		0%			100%	
Search   S	ı	ALΦ					URB/	N ®		
aricade and plants		%	Factor	Cs	Description			%	Factor	$C_2$
Date	Wetlands and pans		0.03	0	Lawns Sandy flat (	79%1			0	0
Dispares   Dispares	Hilly		0.16	0	Sandy, steep	0 (>7%)			0.2	0
Document   Document	Steep areas		0.26	0	Heavy soil, f	lat (<2%)			0.17	0
Part	Total	0%		0	Heavy soil, s	teep (>7%)			0.35	0
permeable	Permeability	%	Factor	င့	Residential	areas				
Documentable   Doc	Very permeable		0.04	0	Houses				0.5	0
Define displanation   Define   Defin	Permeable Semi-permeable		0.08	0	Hats				0.7	0
at etation   bow   control   cont	Impermeable		0.10	0	Light Industr	<u>a</u>		0%	0.7	0
Realization	Total	0%		0	Heavy Indus	trial		100%	0.7	0.7
Control   Con	Vegetation	%	Factor	C <sub>v</sub>	Business					
Control of the period (years), T   Control of th	Thick bush and plantation		0.04	0	City centre				0.95	0
regetation         0.28 0%         0.28 0%         0         Maximum flood         1         0         0.7	Grasslands		0.11	0	Streets				0.95	0
Interpretation   No.	Novegetation		0.28	0	Maximum flo	оd			1	0
	Total	0%		0	Total (C <sub>2</sub> )			100%		0.7
Control   Con	IME OF CONC	ENIKALIO	v (Ic)	ree	Notes:	# H	on hours	So T = 0.35	bolin	
$ \begin{array}{c c c c c c c c c c c c c c c c c c c $	- 15						i c	0		
Control   Con	0.563		0.5	63	3	7	L \0.467	ı	$0.87L^{2}$	0000
RUN-OFF COEFFICIENT   2   25   50   100	10.2409 min	Tc Tc	10.07203	nin	$I_C = I_L$	2,004( <u>√S</u>	av	Ш	1000Sav	)
run period (years), T  2  5  10  20  25  10  20  25  10  20  25  20  27  27  27  27  27  27  27  27  27	_			N-OFF C	OEFFICIEN	7				
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	Return period (years), T			2	5		20	25	50	100
issted for dolomitic areas Cro.         0.7         0.9         1.00           issted run-off coefficient, Crr         0.75         0.56         0.595         0.63         0.63833         0.685         0.7           mbined run-off coefficient, Crr         0.70         0.	Run-off coefficient, $C_1$ ( $C_1 = C_S + C_P + C_V$ )			0.7	0.7	0.7	0.7	0.7	0.7	0.7
Issment factor for initial saturation.         0.75         0.80         0.85         0.90         0.91         0.95         1.00           Issed run-off coefficient, C <sub>1</sub> τ no k F <sub>1</sub> y issed run-off coefficient, C <sub>1</sub> τ         0.525         0.56         0.595         0.63         0.635833         0.665         0.7           C <sub>1</sub> τ βC <sub>2</sub> + VC <sub>2</sub> )         0.70 <td>Adjusted for dolomitic areas,C<sub>1D</sub> (= C<sub>1</sub>(1 - D<sub>%</sub>) + C<sub>1</sub>D<sub>%</sub>(Σ(D<sub>lactor</sub> x C<sub>S</sub>)</td> <td>(()</td> <td></td> <td>0.7</td> <td>0.7</td> <td>0.7</td> <td>0.7</td> <td>0.7</td> <td>0.7</td> <td>0.7</td>	Adjusted for dolomitic areas,C <sub>1D</sub> (= C <sub>1</sub> (1 - D <sub>%</sub> ) + C <sub>1</sub> D <sub>%</sub> (Σ(D <sub>lactor</sub> x C <sub>S</sub> )	(()		0.7	0.7	0.7	0.7	0.7	0.7	0.7
0.525     0.56     0.595     0.63     0.635833     0.665     0.7       0.70     0.70     0.70     0.70     0.70     0.70     0.70     0.70       RAINFALL     20     25     50     100       2     5     10     20     25     50     100       80     107     129     152     157     183     207       100%     100%     100%     100%     100%     100%     100%       80     107     129     152     157     183     207       80     107     129     152     157     183     207       80     107     129     152     157     183     207       80     107     129     152     157     183     207       80     107     129     152     157     183     207       90     5     10     20     25     50     100       1028     531     6.40     7.54     7.78     9.07     10.26	Adjustment factor for initial saturati F <sub>t</sub> ©	on,		0.75	0.80	0.85	0.90	0.91	0.95	1.00
RAINFALL   20   25   50   100   20   25   50   100   20   25   27   27   27   27   27   27   27	Adjusted run-off coefficient, C <sub>1T</sub> (= C <sub>1D</sub> x F <sub>*</sub> )			0.525	0.56	0.595	0.63	0.635833	0.665	0.7
RAINFALL           2         5         10         20         25         50         100           80         107         129         152         157         183         207           100%         100%         100%         100%         100%         100%         100%         20         25         50         100           3397         531         6.40         7.54         7.78         9.07         1026	Coombined run-off coefficiret, C <sub>T</sub>			0.70	0.70	0.70	0.70	0.70	0.70	0.70
2         5         10         20         25         50         100           80         107         129         152         157         183         207           100%         100%         100%         100%         100%         100%         100%         100%           80         107         129         152         157         183         207           2         5         10         20         25         50         100           387         5.31         6.40         7.54         7.78         9.07         10.26	(= αC <sub>1T</sub> + βC <sub>2</sub> + γC <sub>3</sub> )			RA	NFALL					
80 107 129 152 157 183 207 100% 100% 100% 100% 100% 100% 100% 80 107 129 152 157 183 207 2 5 10 20 25 50 100 3,87 5,31 6,40 7,54 7,78 9,07 10,26	Return period (years), T				5	10	20	25	50	100
80         107         129         162         157         183         207           100%         100%         100%         100%         100%         100%         100%         100%           80         107         129         152         157         183         207           2         5         10         20         25         50         100           3,97         5,31         6,40         7,54         7,78         9,07         10,26	Point rainfall (mm), P <sub>T</sub> ®									
RF <sub>1</sub> <sup>(7)</sup> 100%         100%	Point intensity (mm/hour), $P_{lT}$ (= $P_T$	/T <sub>c</sub> )		80	107	129	152	157	183	
. l <sub>T</sub> 80 107 129 152 157 183 <b>2 5 10 20 25 50</b> 3.97 5.31 6.40 7.54 7.78 9.07	Area reduction factor (%), ARF <sub>T</sub> ®			100%	100%	100%	100%	100%	100%	100%
2 5 10 20 25 50 3.97 5.31 6.40 7.54 7.78 9.07	Average intensity (mm/hour), I <sub>T</sub>			80	107	129	152	157	183	207
3.97 5.31 6.40 7.54 7.78 9.07	Return period (years), T			2	υ <sub>1</sub>	10	20	25	50	100
0.00	Peak flow (m <sup>3</sup> /s), Q=CIA/3.6			3.97	5.31	6.40	7.54	7.78	9.07	10.26

Defined watercourse	Define
0.03374778	S (m/m)
19	Н (m)
1351	Hlow (m)
1370	Hhigh (m)
Overland flow	Ov
JLATION	SLOPE CALCULATION
Defined watercourse	Types of How.
Overland flow	Times of flour

		_			
Tyd t (min)	Herhaalings Periode T		t=	Klas =	Stormwater - Standaard Intensiteitskrommes
15	100		15	В	Standaar
15	50				d Intensi
15	25		Tyd waarvoor	Carolina, Cedara,Esto Piet Retief, Potchefstr Sheeprun, Towoomba	eitskrom
15	20		Tyd waarvoor die Intensiteit bepaal word (min)	Carolina, Cedara Estoourt, Jan Smuts, Kokstad, Krugersdorp, Maleking, Pier Reitief, Potchefstroom, Pretoria, Roodeplaat, Rustenburg, Sheepruh, Towoomba	<u>lmes</u>
15	10		epaal word (mi	an Smuts, Kok Pretoria, Rooc	
15	5		.≅	stad, Krugersd deplaat, Ruster	
15	2			orp, Mafeking, burg,	

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Autor Maria
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Star Day

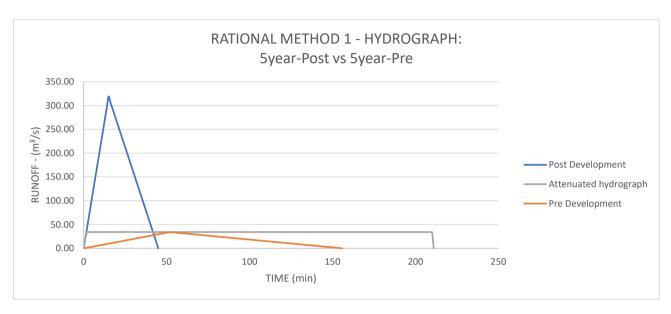
#### **HYDROGRAPH - RATIONAL METHOD 1**

Location:

Date:

2019-094 Lanseria X1 Southern Precinct 28/02/2025

Site 25.5 ha
Tc(Pre-development) 52 min
Tc(Post-development) 15 min
Tc Factor 3



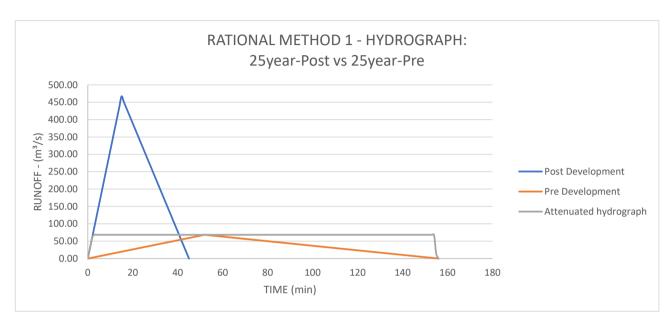
#### **HYDROGRAPH - RATIONAL METHOD 1**

2019-094 Lanseria X1

Location: Southern Precinct 28/02/2025

Date:

Site 25.5 ha Tc(Pre-development) 52 min Tc(Post-development) 15 min Tc Factor 3



Attenuation Pond Volume

7723.24 m<sup>3</sup>

Attenuation

302.8722 m³/ha 1.5 m

(JRA Requirements: 300-350m³/ha)

Attenuation Pond Height Attenuation Pond Area

5148.827 m<sup>2</sup>

## **ANNEXURE J: DEVELOPMENT ACCESS**

